### SIMULATION METHODS FOR WATERSHED MANAGEMENT Discharge

# Topics

- Discharge measurement
- Unit hydrograph method

#### • Purposes:

- Water balance assessment
- Identification and quantification of hydrological extremes
- Waterborne transport
- Hydropower plants
- Ecological assessment

#### Instant (occasional) measurement vs. Continuous (regular intervals) measurement

- There are many methods to measure discharge:
  - Measurement of water level (stage) in a stream channel in combination with the rating curve
    - Mechanical recorders
    - Pressure sensors
    - Ultrasonic sensors
  - Flumes (Venturi flume, Parshal flume)
  - Weirs
  - Measurement of velocities
    - Propeller current meter
    - Acoustic Doppler current meter
  - Chemical tracers (salt dilution)

#### Rating curve

The curve describing stage – discharge relationship. Usually ploted with discharge on the x axis (horizontal) and stage on the y axis (vertical). The curve can be either **measured** or **calculated**.

- Calculated curves are usually based on hydraulic methods application. The accuracy mostly depends on a proper selection of Manning roughness coefficient.
- Measured curves need sufficient amount of measured pairs of stages and discharges. The curve should be considered valid only in range of measured data.





<u>http://www.bdsensors.cz/</u> <u>http://www.bannerengineering.com/en-US/products/sub/498</u>











(Photograph courtesy of Michael Nolan, U.S. Geological Survey)

The current-meter method uses equipment such as (A) the Price AA current meter; (B) the Price AA current meter attached to a wading rod; and (C) the Price AA meter suspended above a heavy weight.



Current-meter discharge measurements are made by determining the discharge in each subsection of a channel cross section and summing the subsection discharges to obtain a total discharge.





#### http://water.usgs.gov/edu/streamflow2.html

Yen-Chang Chen et al. Pollutant Flux Estimation in an Estuary Comparison between Model and Field Measurements. Environments, 2014, 1(1), 107-123. http://www.approtekmooring.com/fixed\_mounting/sidewall\_mounting/SWM/SWM-anim.gif



<u>http://lib.colostate.edu/archives/water/parshall/flume.html</u> <u>https://en.wikipedia.org/wiki/Parshall\_flume#/media/File:Parshall\_Flume.svg</u>

Dr. Ralph Parshall began the development in 1915 by altering Venturi flume





 $Q = C \cdot H_a^n$ 

#### Weirs

- Crest shape: broad crested x sharp crested
- Cross-section shape: triangular (V-notch), rectangular, trapezoidal etc.







#### Broad Crested Weir

http://img.bhs4.com/74/e/74e90e405e86deef <u>b51c67cf425789ee84bfadb8\_large.jpg</u>

http://www.engineeringtoolbox.com/

Length

http://irrigation.wsu.edu/Content/Calculators/Water-Measurements/Rectangular-Contracted-Weir.php





www.engineeringtoolbox.com

# Flood hydrographs

- Stage hydrograph
- Discharge hydrograph

#### Main characteristics of hydrographs

- Peak flow/peak discharge
- Lag time
- Time to peak
- Hydrograph volume
- Hydrograph duration



http://www.slideshare.net/DuncanAshton/hydrogr aph-explanation-and-animation

#### **UH** mtehod

- First concept by Leroy Sherman (1932)
- UH is considered as hypothetic response of the catchment to **effective** rainfall with unit total
- Assumes time-invariancy of transformation process and principle of superposition
- Method is used to transform effective precipitation into runoff hydrograph







Source <u>http://cs.wikipedia.org/wiki/Soubor:Convolution\_TUH.svg</u> Author: Stanislav Horacek

#### **UH** method

The method is based on the superposition of response hydrographs corresponding to each effective precipitation pulse.

$$Q(t) = \sum_{1}^{n} u(t - (i - 1) \cdot \Delta t) \cdot I_{i} \cdot \Delta t$$

$$Q(t) = \int_{0}^{t} u(t-\tau) \cdot I(\tau) \cdot d\tau$$



#### **UH** method

M = number of precipitation pulses (5) X = number of unit hydrograph ordinates (7) n = time step

$$Q_{n} = \sum_{m=1}^{n \le M} P_{m} \cdot U_{n-m+1}$$

$$Q_{1} = \sum_{m=1}^{1} P_{m} \cdot U_{2-m} = P_{1} \cdot U_{1}$$

$$Q_{2} = \sum_{m=1}^{2} P_{m} \cdot U_{3-m} = P_{1} \cdot U_{2} + P_{2} \cdot U_{1}$$

$$Q_{3} = \sum_{m=1}^{3} P_{m} \cdot U_{4-m} = P_{1} \cdot U_{3} + P_{2} \cdot U_{2} + P_{3} \cdot U_{1}$$

$$Q_{4} = \sum_{m=1}^{4} P_{m} \cdot U_{5-m} = P_{1} \cdot U_{4} + P_{2} \cdot U_{3} + P_{3} \cdot U_{2} + P_{4} \cdot U_{1}$$

$$Q_{10} = \sum_{m=1}^{5} P_m \cdot U_{11-m} = P_1 \cdot U_{10} + P_2 \cdot U_9 + P_3 \cdot U_8 + P_4 \cdot U_7 + P_5 \cdot U_6 = P_4 \cdot U_7 + P_5 \cdot U_6 + P_6 \cdot U_5$$
$$U_{10}, U_9, U_8 = 0$$

#### UH method - overview

- Representation of natural catchment by linear system is only approximative and relevance of used principles is limited
- Method doesn't consider flow transformation in the stream channel which can vary
- Method assumes spatialy homogeneous rainfall
- Ordinates of unit hydrograph can be calculated from measured rainfall-runoff data
- Parameters of unit hydrograph are estimated or calculated using catchment properties for ungauged catchments (SCS method, Clark's method, Snyders method etc.)