



# Groundwater hydraulics and modelling - GWHM

## Excercise 1 – Darcy's law

**Martina Sobotková, B609**

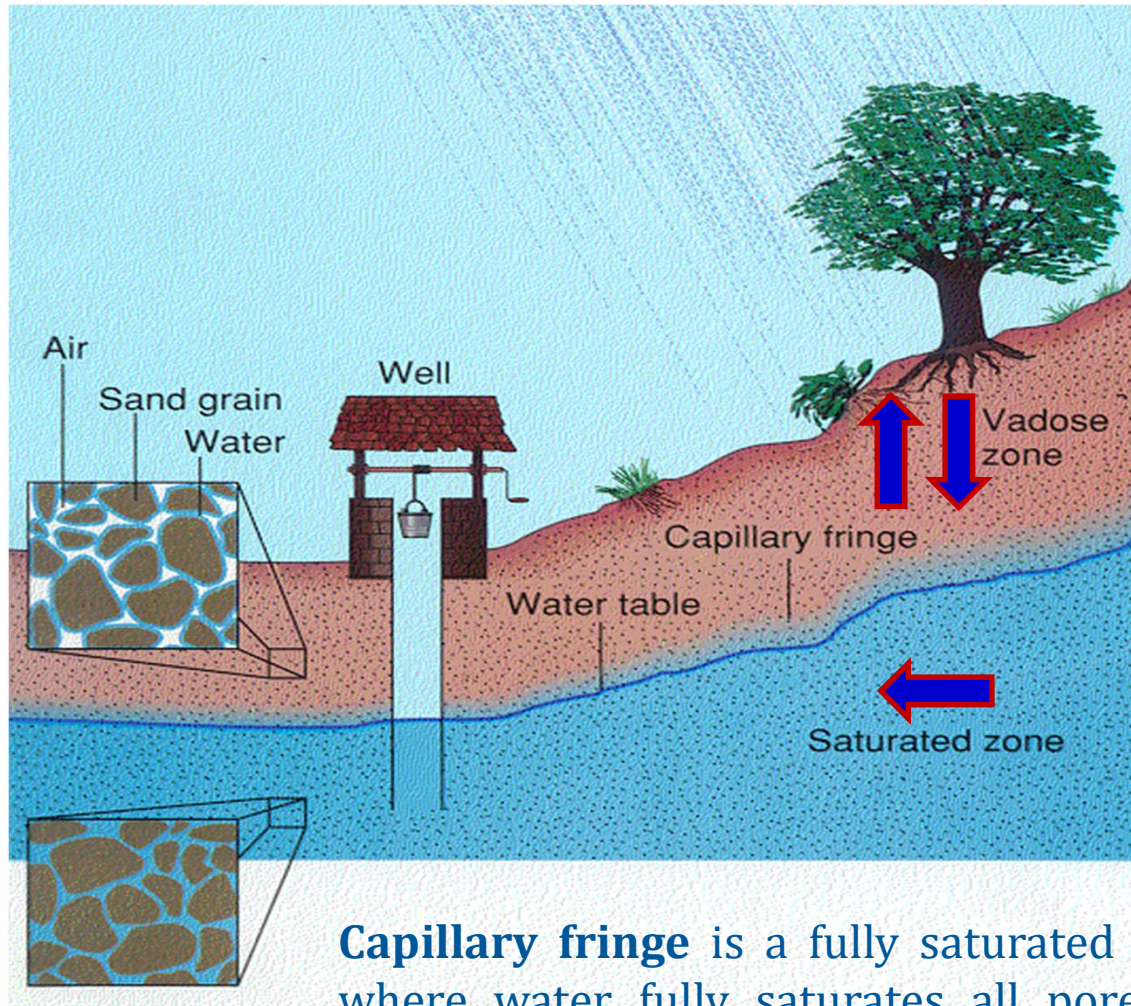
[martina.sobotkova@fsv.cvut.cz](mailto:martina.sobotkova@fsv.cvut.cz)

**Dept. of Irrigation, Drainage and Landscape management**

# Themes of excercises

- 1. Terminology - saturated and unsaturated zone, porosity, hydraulic conductivity**
- 2. Darcy's law - examples**

# Groundwater in hydrological cycle



Soil above the water table:  
solids – water - air  
**UNSATURATED ZONE**

Soil below water table:  
solids - water  
**SATURATED ZONE**

Flow directions:  
**Vadose zone** ↑ ↓  
**Saturated zone** →

**Capillary fringe** is a fully saturated zone above the groundwater where water fully saturates all pores thanks to capillary forces greater in absolute than capillary pressure relevant for the biggest pore (see definition of *air entry value* in soil physics)

# Porosity, saturated hydraulic conductivity

Pore volume:  $V = V_s + V_p$

↑                    ↑  
volume of solids volume of voids

Porosity:  $n = \frac{V_p}{V}$

← volume of voids  
← total volume

**Void ratio:** defined as the ratio between the volume of voids and the volume of solids:

$$e = \frac{V_p}{V_s}$$

**Hydraulic conductivity:** indicates the ability of the aquifer to conduct water through it

$$K \dots [\text{m/s}] \quad v = K * I$$

**Saturated hydraulic conductivity:**  $K_s \dots [\text{m/s}]$

$$K_s = 1 * 10^{-4} \text{ [m/s]} \dots \textit{coarse sand}$$

$$K_s = 1 * 10^{-11} \text{ [m/s]} \dots \textit{clay}$$

# Groundwater movement

Flow potential for incompressible fluid ( $\rho = \text{const.}$ ):  $\Phi = gz + \frac{p - p_0}{\rho}$

Water pressure is typically assigned at zero level equal to atmospheric pressure  $p_0$ , then

flow potential is defined as

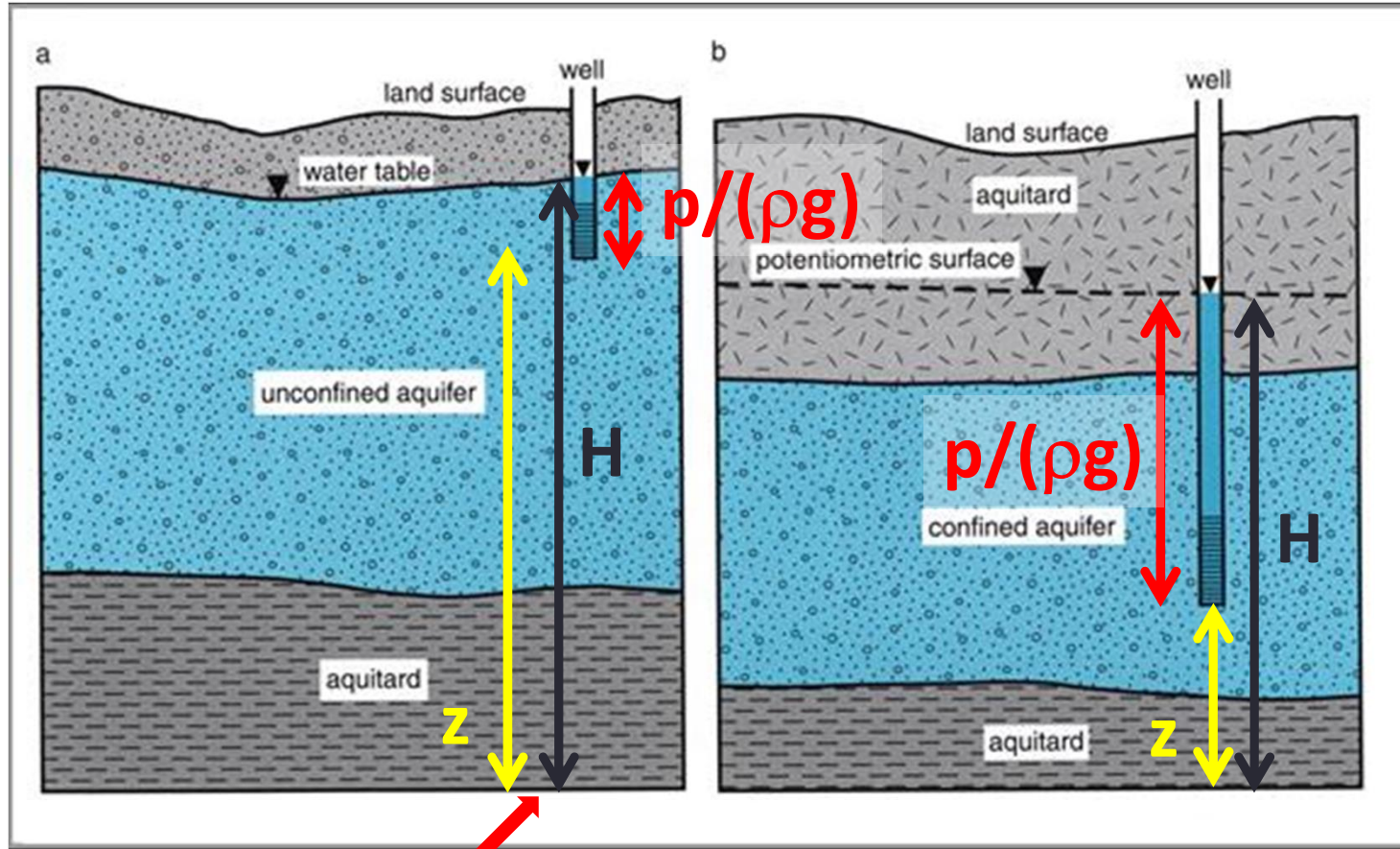
$$\Phi = gz + \frac{p}{\rho}$$

Water flow in between two points in porous space occurs in case of different potentials of water and simultaneously, there is continuous passage permeable for water. Water moves from level of higher to lower potential.

Groundwater hydraulics typically describes potential as **hydraulic head H**

$$H = \frac{\Phi}{g} \quad H = z + \frac{p}{\rho g}$$

# Measurement of hydraulic head

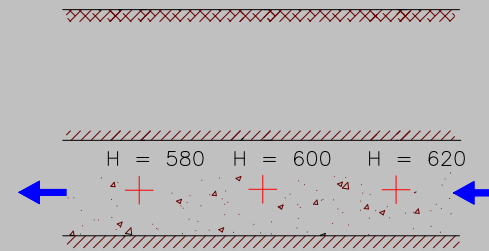
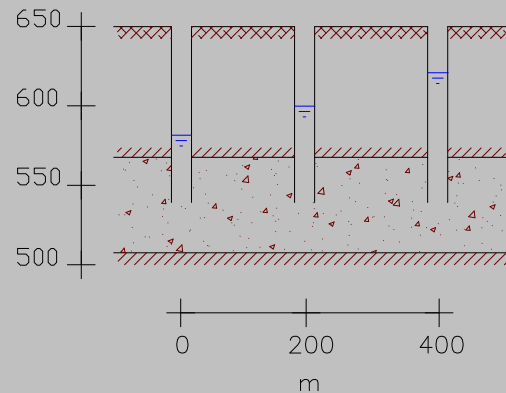


$z=0$

$$H = \frac{\Phi}{g} \quad H = z + \frac{p}{\rho g}$$

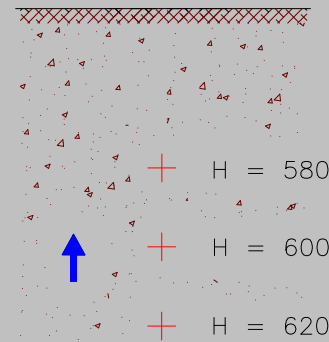
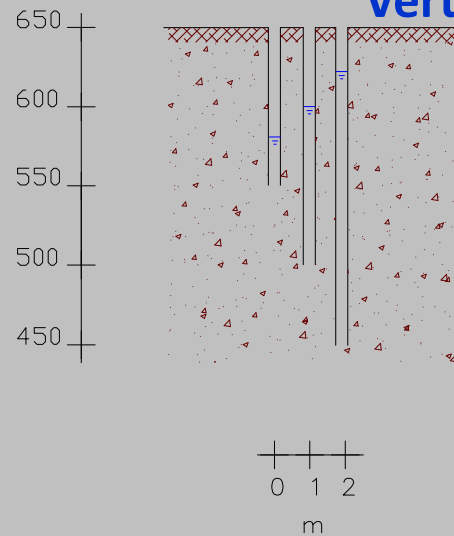
# Determination of groundwater flow direction based on group of piezometers

## Horizontal direction of flow



$$\frac{dH}{dL} = \frac{20}{200} = 0.10$$

## Vertical flow direction



$$\frac{dH}{dL} = \frac{20}{50} = 0.40$$

# Saturated flow

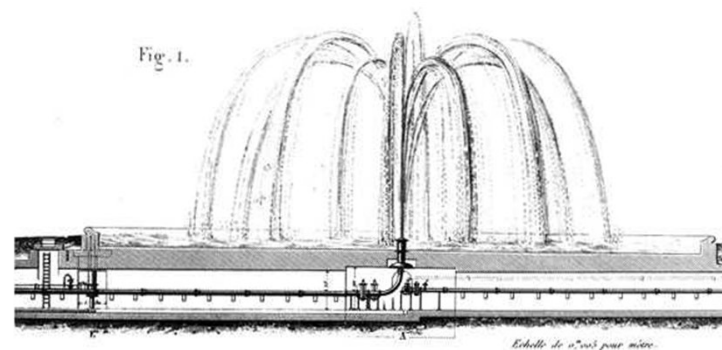
**Henri Darcy (1856)** filtration of water for fountains in Dijon

After many experiments he found that **water flow** through the soil column depends on:

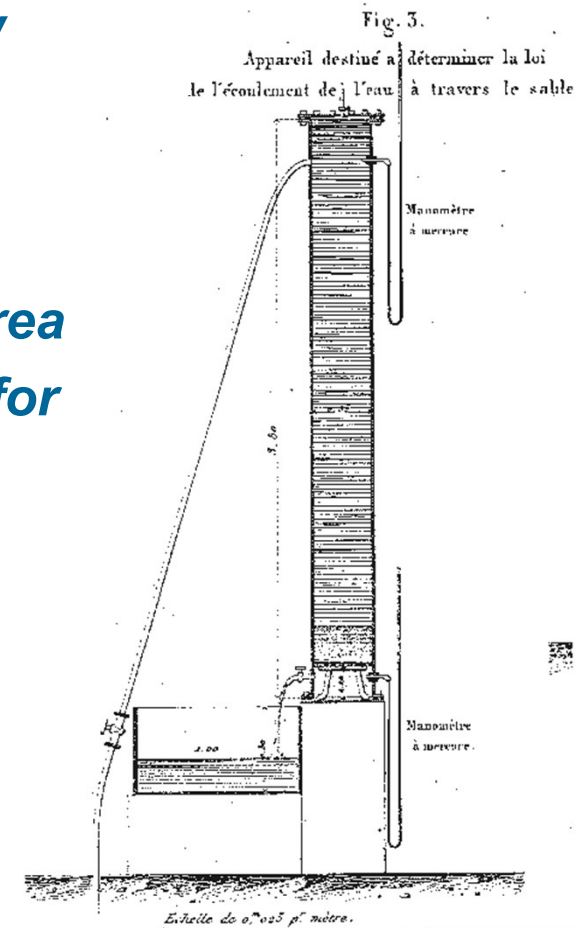
- **directly proportional to pressure drop**
- **inversely proportional to the length**
- **directly proportional to the crosssectional area**
- **dependent on coefficient which is specific for each media**



Henry Darcy



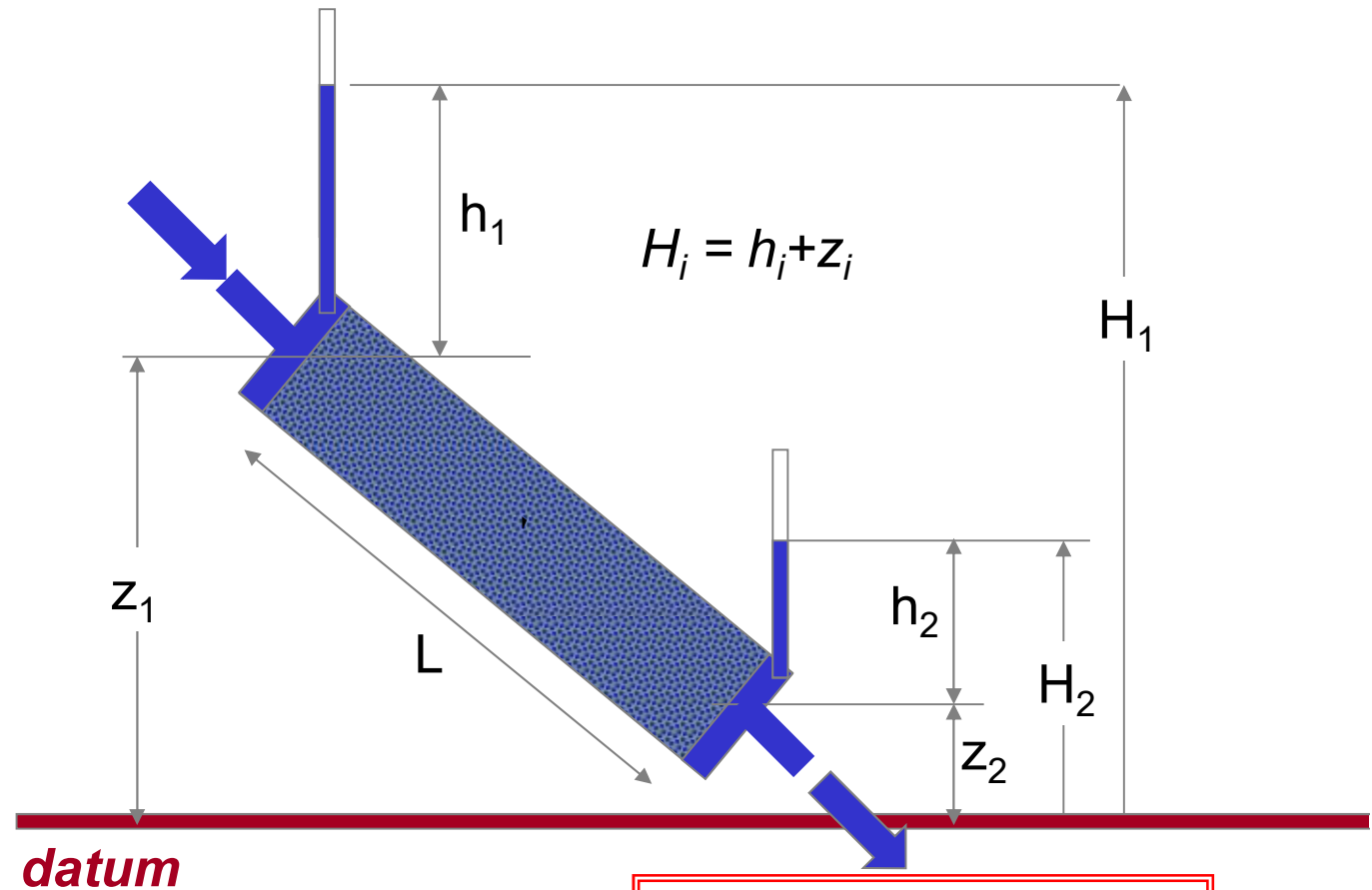
Darcy, H., 1856. *Les Fontaines de la Ville de Dijon*





# Darcy law

$$Q = \frac{K_s A \Delta H}{L}$$



$Q = \text{flow } [L^3 \cdot T^{-1}]$

$A = \text{crosssectional area } [L^2]$

$K_s = \text{saturated hydraulic conductivity } [L \cdot T^{-1}]$

$\Delta H = H_1 - H_2$  (hydraulic head drop)  $[L]$

$L = \text{sample length } [L]$

**valid in fully  
saturated porous  
media  
For example: under  
the ground water  
level**

for:

$$q = \frac{Q}{A}$$

kde:

$q$  ... Volume flux [ $L \cdot T^{-1}$ ]

$Q$  ... Flow rate [ $L^3 \cdot T^{-1}$ ]

$A$  ... Crosssectional area [ $L^2$ ]

Transforms to the:

$$q = K_s \frac{\Delta H}{L}$$

More general form:

$$q = K_s \frac{dH}{dl}$$

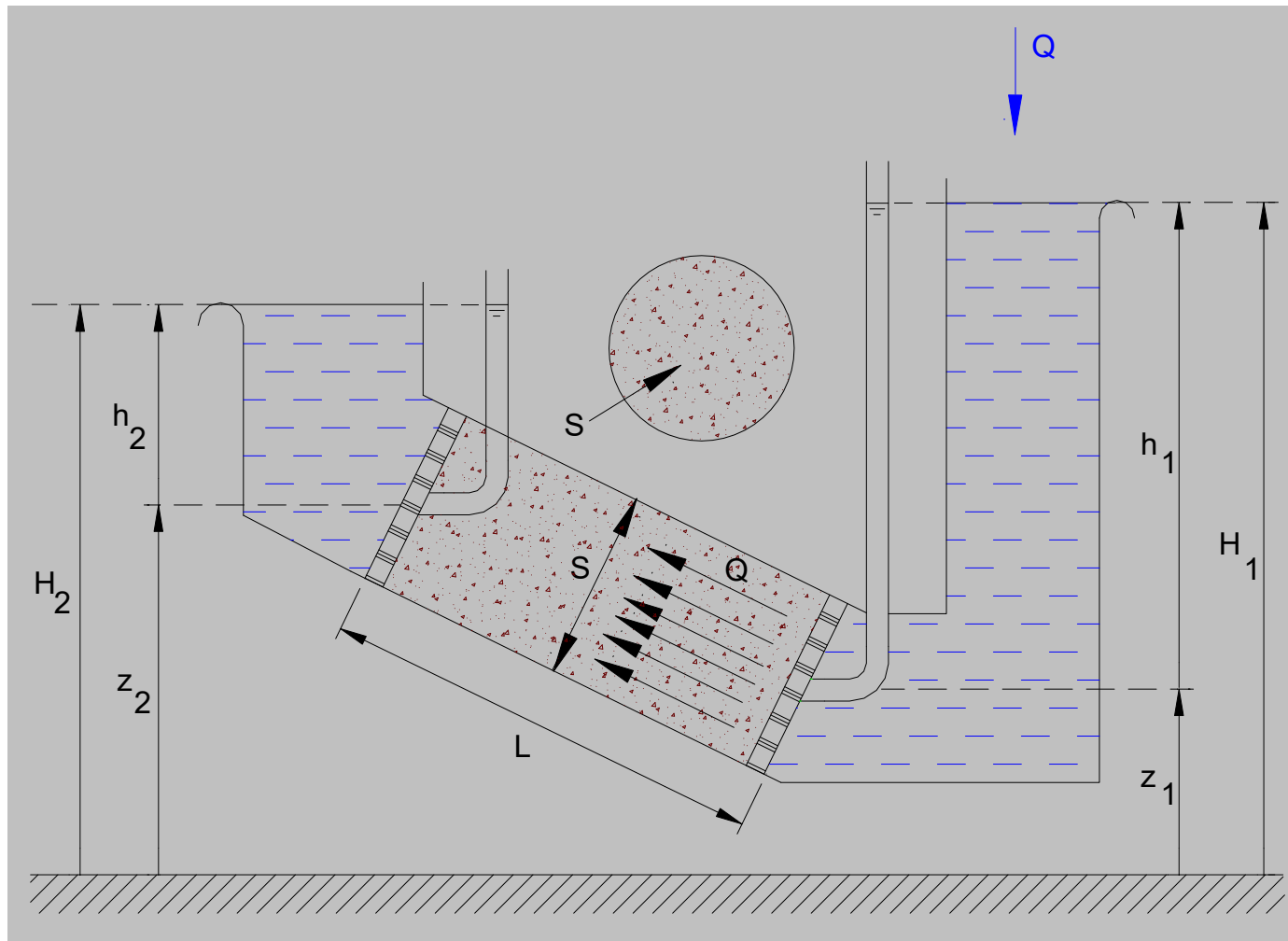
**note: negative sign  
due to the fact  $\text{grad } H$   
aims against flow  
direction**

Pro 1D vertical flow

$$q = -K_s \frac{dH}{dl} = -K_s \nabla H$$

# Example 1 – Darcy law

Henri Darcy - 1856



$$Q = S * K * I$$

FLOW AREA (pointing to S)  
 HYDRAULIC GRADIENT (pointing to I, circled in red)  
 FLOW (pointing to Q)  
 HYDRAULIC CONDUCTIVITY (pointing to K)

$$v = \frac{Q}{S}$$

$$v_p = \frac{Q}{S_n}$$

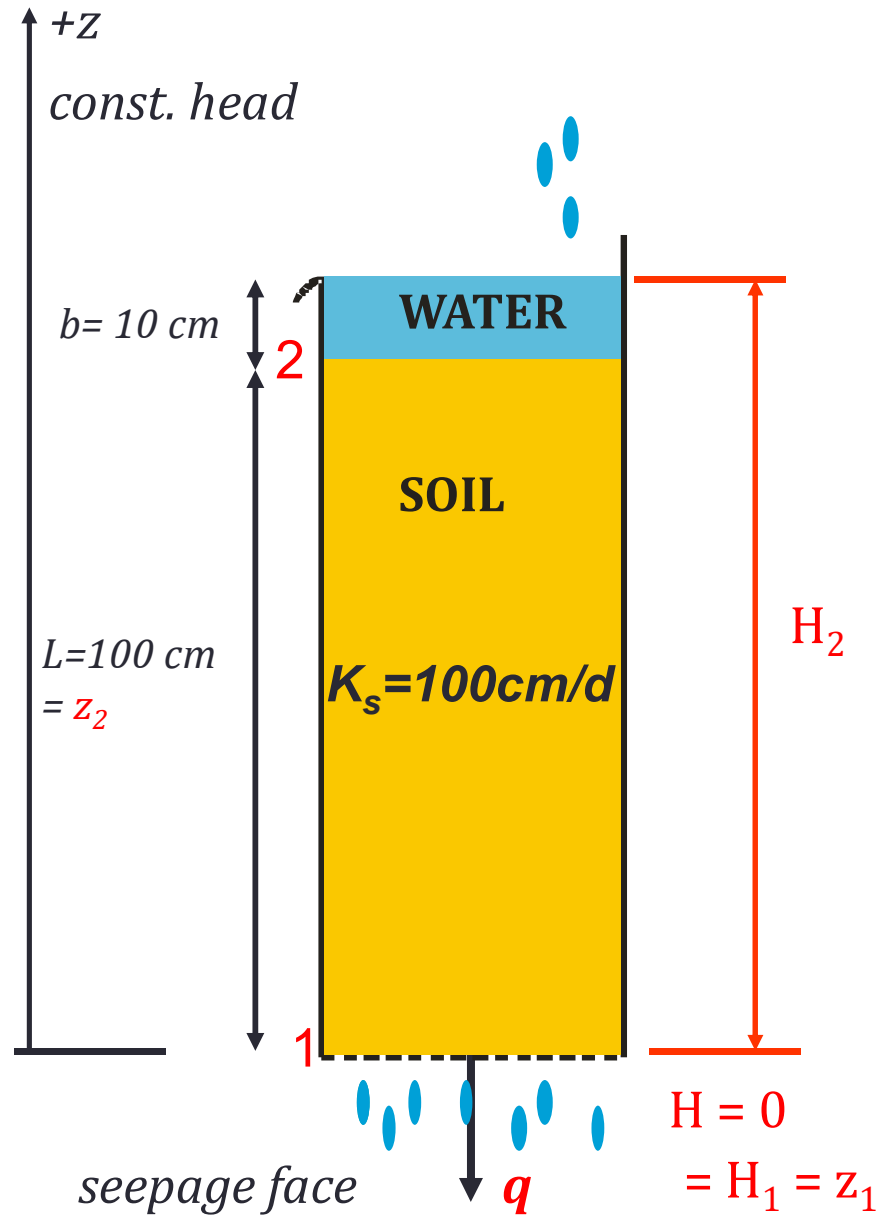
$$H = z + \frac{p}{\rho * g}$$

$$I = \frac{H_1 - H_2}{L}$$

**Bernoulli's equation**  $z_1 + \frac{p_1}{\rho g} + \frac{v_1}{2g} = z_2 + \frac{p_2}{\rho g} + \frac{v_2}{2g}$

# Example 1

Find  $q=?$



## Example 1

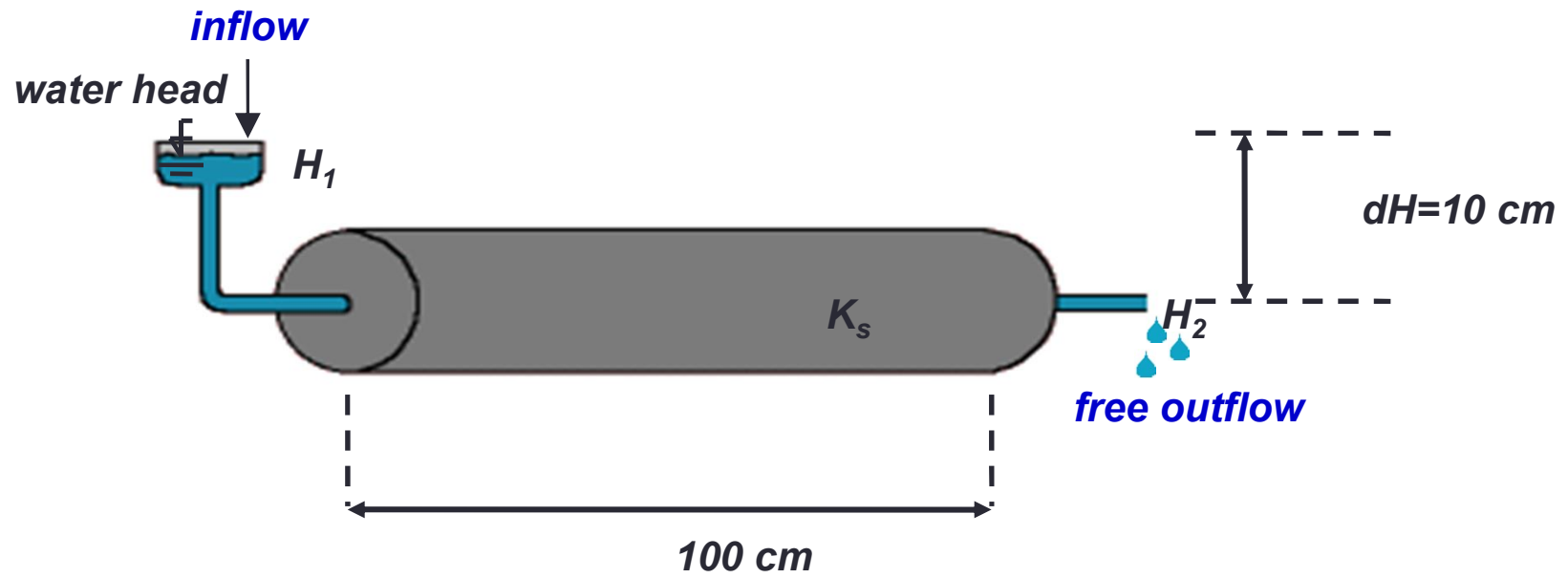
1) datum definition

2) points 1 and 2 with known hydraulic heads

3) Darcy's law

$$q = -K_s \frac{\Delta H}{L} = -K_s \frac{(H_2 - H_1)}{L} = -100 \frac{(110 - 0)}{100} = -\mathbf{110 \text{ cm.d}^{-1}}$$

## Example 2



## Example 2

1) Step 1 - definition of *coordination system*

2) Definition of points 1 and 2).

Then  $x_1 = 0$  and  $h_1 = 10$  cm,  $x_2 = 100$  cm,  $h_2 = 0$ ,  $z_1 = z_2 = 0$ ,  $L = x_2 - x_1 = 100$  cm

3) Hydraulics heads are then:  $H_1 = h_1 + z_1 = 10$  cm,  $H_2 = h_2 + z_2 = 0$  cm

4) Darcy's law

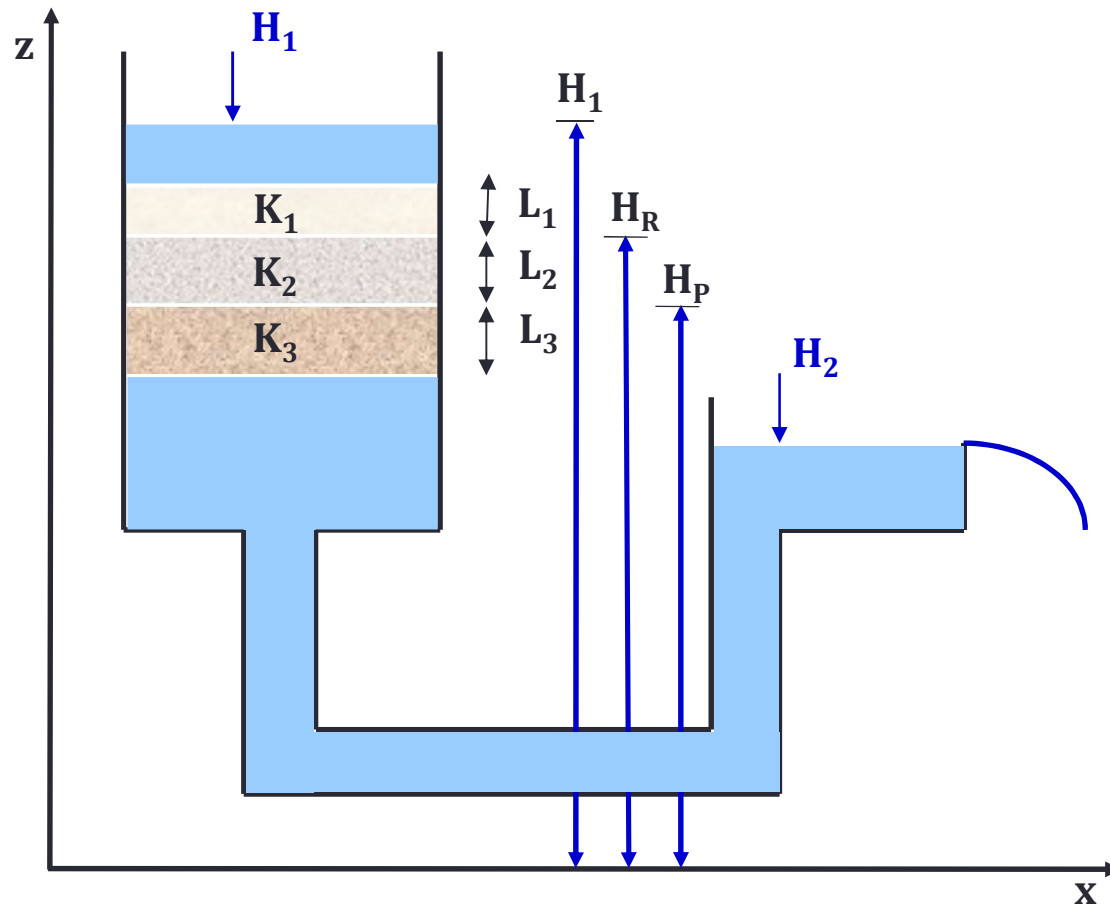
$$q = -K_s \frac{\Delta H}{L} = -K_s \frac{(H_2 - H_1)}{L} = -100 \frac{(0 - 10)}{100} = 10 \text{ cm.d}^{-1}$$



### Example 3

Calculate  $Q = ?$  ( $\text{m}^3/\text{s}$ ) with known saturated hydraulic conductivities of three different materials  $K_1 = 0.001$  m/s,  $K_2 = 0.01$  m/s,  $K_3 = 0.01$  m/s. Hydraulic heads are defined as:  $H_1 = 1.2$  m,  $H_R = 0.7$  m,  $H_p = 0.6$  m, and  $H_2 = 0.4$  m. Column area is  $S = 0.0003$   $\text{m}^2$  and heights of each porous media are  $L_1 = 0.1$  m,  $L_2 = 0.2$  m, and  $L_3 = 0.4$  m.

# Example 3



### Example 3

$$Q = Q_1 = Q_2 = Q_3$$

$$Q_1 = K_1 S \frac{H_1 - H_R}{L_1} = 0.001 * 0.0003 * \frac{1.2 - 0.7}{0.1} = 1.5 * 10^{-6} \text{ m/s}$$

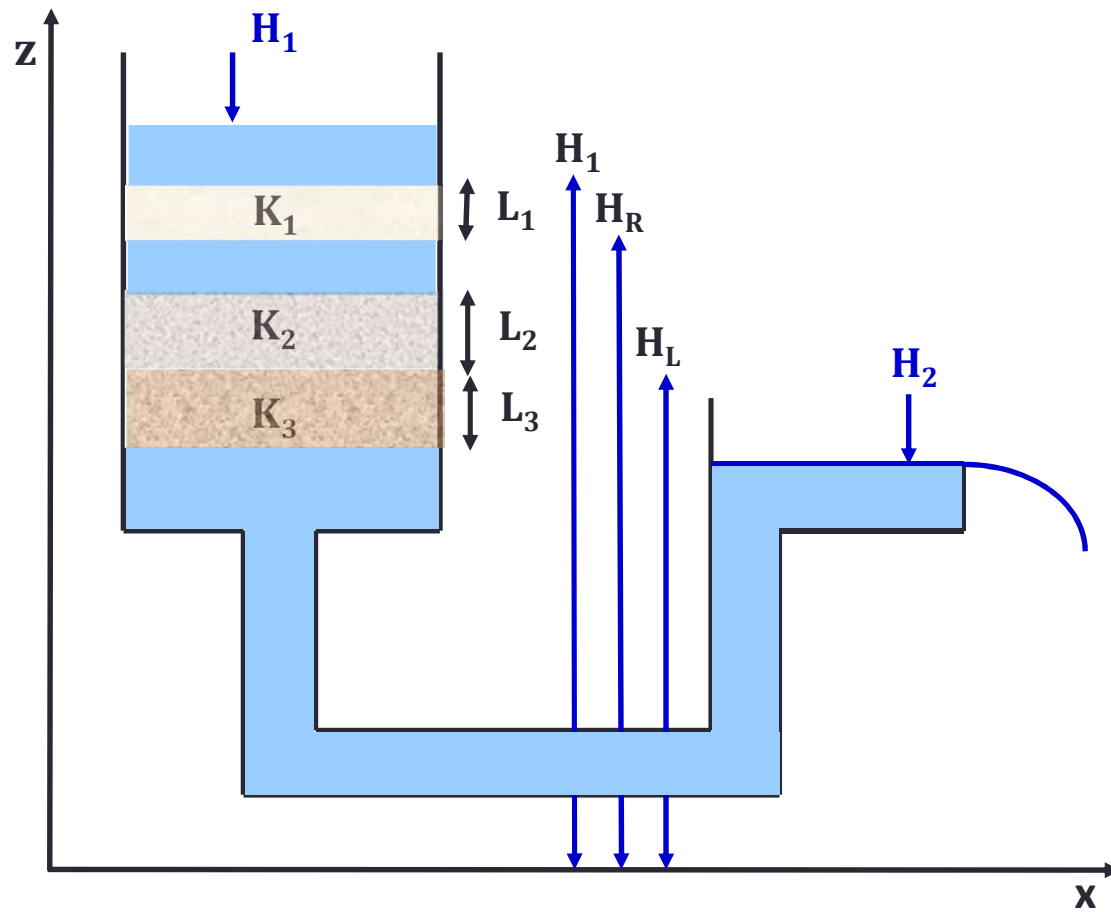
$$Q_2 = K_2 S \frac{H_R - H_P}{L_2} = 0.01 * 0.0003 * \frac{0.7 - 0.6}{0.2} = 1.5 * 10^{-6} \text{ m/s}$$

$$Q_3 = K_3 S \frac{H_P - H_2}{L_3} = 0.01 * 0.0003 * \frac{0.6 - 0.4}{0.4} = 1.5 * 10^{-6} \text{ m/s}$$

### Example 3

Calculate  $Q = ?$  ( $\text{m}^3/\text{s}$ ) with known saturated hydraulic conductivities of three different materials  $K_1 = 0.001$  m/s,  $K_2 = 0.01$  m/s,  $K_3 = 0.01$  m/s. Hydraulic heads are defined as:  $H_1 = 1.2$  m,  $H_R = 0.7$  m,  $H_L = 0.6$  m, and  $H_2 = 0.4$  m. Column area is  $S = 0.0003$   $\text{m}^2$  and heights of each porous media are  $L_1 = 0.1$  m,  $L_2 = 0.2$  m, and  $L_3 = 0.4$  m.

# Example 4



### Example 3

$$Q = Q_1 = Q_2 = Q_3$$

$$Q_1 = K_1 S \frac{H_1 - H_R}{L_1} = 0.001 * 0.0003 * \frac{1.2 - 0.7}{0.1} = 1.5 * 10^{-6} \text{ m/s}$$

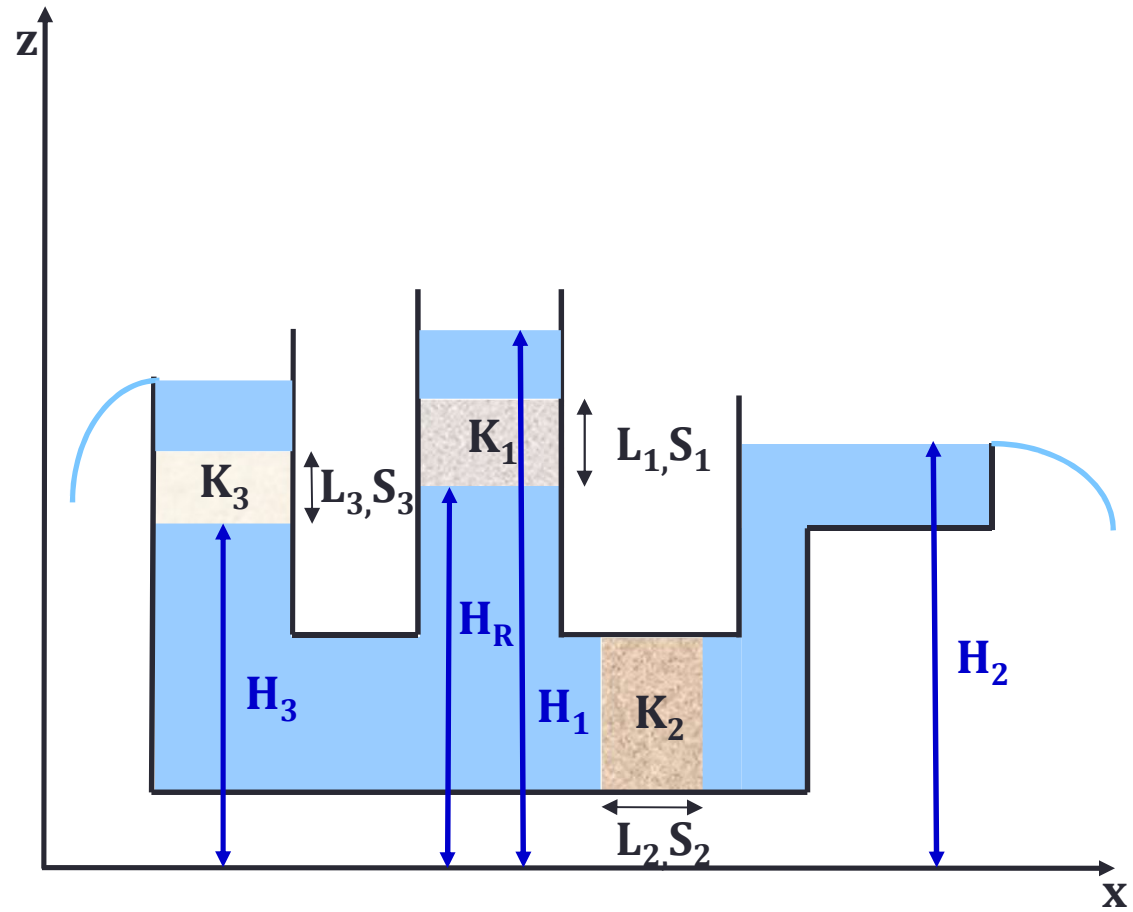
$$Q_2 = K_2 S \frac{H_R - H_L}{L_2} = 0.01 * 0.0003 * \frac{0.7 - 0.6}{0.2} = 1.5 * 10^{-6} \text{ m/s}$$

$$Q_3 = K_3 S \frac{H_L - H_2}{L_3} = 0.01 * 0.0003 * \frac{0.6 - 0.4}{0.4} = 1.5 * 10^{-6} \text{ m/s}$$

## Example 5

Calculate  $Q_1 = ?$ ,  $Q_2 = ?$ ,  $Q_3 = ?$  m<sup>3</sup>/s with known saturated hydraulic conductivities of three different materials  $K_1 = 0.05$  m/s,  $K_2 = 0.1$  m/s,  $K_3 = 0.04$  m/s. Hydraulic heads are defined as:  $H_1 = 2.0$  m,  $H_R = 1.5$  m,  $H_2 = 1.3$  m, and  $H_3 = 1.3$  m. Column area is  $S_1 = S_2 = S_3 = 0.79$  m<sup>2</sup> and heights of each porous media are  $L_1 = 0.020$  m,  $L_2 = 0.022$  m, and  $L_3 = 0.022$  m.

# Example 5





## Example 5

$$Q_1 = Q_2 + Q_3$$

$$Q_1 = K_1 S_1 \frac{H_1 - H_R}{L_1} = 0.05 * 0.79 * \frac{2.0 - 1.5}{0.02} = 1 \text{ m/s}$$

$$Q_2 = K_2 S_2 \frac{H_R - H_2}{L_2} = 0.1 * 0.79 * \frac{1.5 - 1.3}{0.022} = 0.7 \text{ m/s}$$

$$Q_3 = K_3 S_3 \frac{H_R - H_3}{L_3} = 0.04 * 0.79 * \frac{1.5 - 1.3}{0.022} = 0.3 \text{ m/s}$$

## Example 5

Calculate  $Q_1 = ?$ ,  $Q_2 = ?$ ,  $Q_3 = ?$  m<sup>3</sup>/s with known saturated hydraulic conductivities of three different materials  $K_1 = 4.35$  m/s,  $K_2 = 0.80$  m/s,  $K_3 = 0.90$  m/s. Hydraulic heads are defined as:  $H_1 = 3.0$  m,  $H_R = 2.1$  m,  $H_2 = 1.0$  m. Column area is  $S_1 = S_2 = S_3 = 2.54$  m<sup>2</sup> and heights of each porous media are  $L_1 = 1.0$  m,  $L_2 = 0.45$  m, and  $L_3 = 0.50$  m.

# Example 6

